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Numerical experiments on the performance of the RBF meshfree Galerkin Methods for solid mechanics

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Abstract—In this work the advances in meshfree methods, particularly the Radial Basis Function based meshfree Galerkin Methods, are presented with the purpose of analyzing the performance of their meshless approximations and integration techniques. The Radial Point Interpolation Method (RPIM) is studied based on the global Galerkin weak form performed using classical Gaussian integration and the stabilized conforming nodal integration scheme. The numerical performance of this category of methods is tested on their behavior on two elastic problems with regular node grids, and two other with distorted irregular grids. All RPIM methods perform very well in term of elastic computation, the Smoothed Radial Point Interpolation Method (SRPIM) shows a higher accuracy, especially in a situation of distorted node schemes. Keywords—Radial Basis Function, Galerkin weak form, Nodal Integration.

I. INTRODUCTION

One of the most important progress in the field of the numerical simulation was the development of the finite element method (FEM). In this method, a continuum solid defined by an infinity of material points is divided into finite elements which are connected between them by a kind of “grid”. The finite element method (FEM) proved to be effective and robust in several engineering fields because of its capacity to deal with complex geometries. However, it remains that this method suffers from some limitations related to the use of meshes, especially when severe element distortions take place under large deformation processes where the accuracy in FEM results are considerably lost [1]. To surmount these problems, numerical methods known usually as “meshless” or “meshfree” methods were proposed, in these methods the problem domain is represented by a set of scattered nodes, without the need of any, a priori, information on the relationship between them. The development of some of the meshless methods goes back to more than seventy years, with the appearance of collocation methods [2] [3] [4]. After that, the first well known meshless method: the Smoothed Particle Hydrodynamics (SPH) [5], was originally created for the simulation of astrophysical phenomena by Lucy [7], and from the early 1990s, numerous methods have been proposed; for instance the diffuse element method (DEM) [8], the reproducing kernel particle method (RKPM) [9] [10], the element free Galerkin (EFG) method [11], the point interpolation methods [12], the meshless local Petrov-Galerkin method (MLPG) [13]. These methods use “Meshless” shape functions to represent the field variables, since these shape functions are mathematically constructed by using only a set of nodes without requiring a mesh.

The Moving least square (MLS) interpolation was one of the first shape functions used by Belytschko et al. [14] for the development of the element free Galerkin (EFG) method, and because of the limitations which suffered this method, in particular, the complexity of the calculations of MLS shape functions and their partial derivatives, besides the difficulty to imposing boundary conditions [15], Liu and Gu [16], [17] proposed a new family of meshless shape functions, that they called "Point Interpolation Methods". Among these methods, the radial point interpolation method (RPIM) is preferred because the use of radial basis function avoids us falling in the singularity problem of the conventional PIM [18] [45], and shape functions resulting from RBF are stable and hence flexible for arbitrary and irregular nodal configurations. For the achievement of a numerical simulation for mechanics problems we need in combination with shape function a formulation procedure based on strong or weak-forms derived directly from the physical principles. In general, we use weak-form formulations to construct discretized system equations, and the most widely used approach is the Galerkin weak-forms.

For the requirement of a weaker consistency on the approximate function, weak forms need an integral operation performed numerically by the use of two major techniques, the classical Gauss integration and the stabilized conforming nodal integration (SCNI) proposed by Beissel and Belytschko [19] and after by Chen et al. [20], [21].

In the present work, our objectif is to study the RBF meshfree Galerkin Methods through their performances in term of: interpolations (RPIM shape function) and their numerical integration techniques (classical Gauss integration and the stabilized conforming nodal integration).

II. CONSTRUCTION OF RPIM SHAPE FUNCTIONS

The interpolation employed for the construction of the RPIM shape functions augmented with polynomials can be
written as: \[ u(x) = \sum_{i=1}^{n} r_i(x) a_i + \sum_{j=1}^{m} p_j(x) b_j = r^T(x) a + p^T(x) b \quad (1) \]

Where \( r_i(x) \) is a radial basis function (FRB), \( p_j(x) \) is a basis function of monomials \([x, y] \) in 2D problems. Coefficients \( a_i \) and \( b_i \) are the corresponding constants yet to be determined, \( n \) is the number of RBFs, \( m \) is the number of polynomial basis functions.

To find out coefficients \( a_i \) et \( b_i \), we have to satisfy equation (1) at the \( n \) nodes in the local support domain of the point of interest at \( x \), this leads to \( n \) linear equations, then the matrix form of these equations can be written as:

\[ u = R_0 a + P_m b \quad (2) \]

where:

\[ u = \{u_1 \ u_2 \ u_3 \ \cdots \ u_n\}^T \]

the FRB matrix:

\[ R_0 = \begin{bmatrix} r_1(d_1) & r_2(d_1) & \cdots & r_n(d_1) \\ r_1(d_2) & r_2(d_2) & \cdots & r_n(d_2) \\ \vdots & \vdots & \ddots & \vdots \\ r_1(d_n) & r_2(d_n) & \cdots & r_n(d_n) \end{bmatrix}_{(n \times n)} \quad (4) \]

Where \( d_i \) in \( r_i(d_k) \) is defined as:

\[ d_k = \sqrt{(x_k - x_i)^2 + (y_k - y_i)^2} \quad (5) \]

and the Polynomial basis functions matrix:

\[ P_m = \begin{bmatrix} 1 & x_1 & y_1 & \cdots & p_m(x_1) \\ 1 & x_2 & y_2 & \cdots & p_m(x_2) \\ \vdots & \vdots & \vdots & \ddots & \vdots \\ 1 & x_n & y_n & \cdots & p_m(x_n) \end{bmatrix}_{(m \times n)} \quad (6) \]

in this case, there are \( n + m \) variables in Eq(2) so an other \( m \) equations should be required. Golberg et al. (22) added the additional \( m \) equations by using the following constraint conditions:

\[ \mathbf{P}_m^T \mathbf{a} = \sum_{i=1}^{n} p_j(x_i) a_i = 0, \quad j = 1, 2, \ldots, m \quad (7) \]

the vector that collect coefficients for RBFs is:

\[ a^T = \{ a_1 \ a_2 \ \cdots \ a_n \} \quad (8) \]

the vector that collect coefficients for Polynomial basis functions is:

\[ b^T = \{ b_1 \ b_2 \ \cdots \ b_m \} \quad (9) \]

the equation (2) can be written in the following form:

\[ \tilde{u} = \begin{bmatrix} u \\ 0 \end{bmatrix} = \begin{bmatrix} R_0 & P_m \\ \mathbf{P}_m^T & 0 \end{bmatrix} \begin{bmatrix} a \\ b \end{bmatrix} = \mathbf{G} \mathbf{a}_0 \quad (10) \]

Since the matrix \( \mathbf{R}_0 \) is symmetric, the matrix \( \mathbf{G} \) will also be symmetric, than by solving equation (10), we obtain:

\[ \mathbf{a}_0 = \{ a \ b \} = \mathbf{G}^{-1} \tilde{u} \quad (13) \]

The RPIM shape function is finally expressed as

\[ u(x) = \{ r^T(x) \ p^T(x) \} \{ a \ b \} \quad (14) \]

\[ u(x) = \{ r^T(x) \ p^T(x) \} \mathbf{G}^{-1} \tilde{u} = \phi^T(x) \tilde{u} \quad (15) \]

\[ \phi^T(x) = \{ \phi_1(x) \ \phi_2(x) \ \cdots \ \phi_n(x) \} \quad (17) \]

Where the RPIM shape functions corresponding to the nodal displacements are given by

\[ \phi^T(x) = \{ \phi_1(x) \ \phi_2(x) \ \cdots \ \phi_n(x) \} \quad (18) \]

It can be seen that the resultant RPIM shape function has the delta Kronecker property and partition of unity, and due to the addition of polynomial basis, they also fulfill the reproducing properties. In this work, different radial basis functions augmented with the linear polynomial basis are used to construct the present RPIM shape function, the choice of the shape parameters in the RBFs are studied.

III. GALERKIN WEAK FORM OF 2-D SOLID MECHANICS

A 2-D problem of solid mechanics defined in the domain \( \Omega \) bounded by \( \Gamma \) can be described by the following equilibrium equation:

\[ \nabla \cdot \sigma + \mathbf{F} = 0 \quad \text{in} \ \Omega \quad (19) \]

where \( \sigma \) is the Cauchy stress tensor and \( \mathbf{F} \) the body forces vector. The boundary conditions for the equilibrium equations are:

\[ \mathbf{n} \cdot \sigma = \mathbf{i} \quad \text{on the natural boundary} \ \Gamma_t \quad (20) \]

\[ \mathbf{u} = \mathbf{u}_0 \quad \text{on the essential boundary} \ \Gamma_u \quad (21) \]

where \( \mathbf{u} \) is a prescribed displacement on boundary \( \Gamma_u \), \( \mathbf{i} \) is a prescribed traction on the boundary \( \Gamma_t \) and \( \mathbf{n} \) is the outward normal on the boundary.

The well-known Galerkin weak form is given by:

\[ \int_{\Omega} \delta (\nabla \mathbf{u}^T) \cdot \sigma \ d\Omega - \int_{\Gamma_t} \delta \mathbf{u}^T \cdot \mathbf{F} d\Omega - \int_{\Gamma_t} \delta \mathbf{u} \cdot \mathbf{t} d\Gamma = 0 \quad (22) \]

Discretization of Eq. (22) with interpolation function Eq. (15) yields

\[ \mathbf{K} \mathbf{u} = \mathbf{f} \quad (23) \]
where

\[ K_{ij} = \int_{\Omega} B_i^T C B_j \, d\Omega, \quad \text{and} \quad f_i \]  

\[ = \int_{\Omega} \Phi_i^T F \, d\Omega + \int_{\Gamma_s} \Phi_i^T \tilde{t} \, d\Gamma \quad \text{(24)} \]

where \( C \) is the matrix of elastic constants and \( B_i \) is the strain matrix.

The integrals involved in Eq. (24) are usually evaluated numerically through the well known Gauss integration scheme as is commonly used in finite elements. In this study, a second scheme is also used: the stabilized conforming-in-ignodal integration scheme (SCNI). Both schemes are shortly explained below.

IV. INTEGRATION TECHNIQUES

A. Gauss integration

In order to evaluate integrals over the global problem domain \( \Omega \) and the global traction boundary \( \Gamma_t \), the problem domain is discretized into a set of background cells. Hence, a global integration can be expressed as:

\[ \int_{\Omega} G \, d\Omega = \sum_{k=1}^{n_c} \int_{\Omega_k} G \, d\Omega = \sum_{k=1}^{n_c} \sum_{i=1}^{n_g} \tilde{w}_i \, G(\mathbf{x}_{Qi}) \, |J_{ik}| \quad \text{(25)} \]

where \( n_c \) is the number of background cells, \( n_g \) is the number of Gauss points used in a background cell, \( G \) represents the integrand, \( \Omega_k \) is the domain of the \( k^{th} \) background cell, \( \tilde{w}_i \) is the Gauss weighting factor for the \( i^{th} \) Gauss point at \( \mathbf{x}_{Qi} \), and \( J_{ik} \) is the Jacobian matrix for the area integration of the background cell \( k \).

Similarly, we can obtain the formulation of the curve Gauss quadrature as:

\[ \int_{\Gamma} G \, d\Gamma = \sum_{l=1}^{n_{ct}} \int_{\Gamma_{lt}} G \, d\Omega = \sum_{l=1}^{n_{ct}} \sum_{i=1}^{n_{gt}} \tilde{w}_i \, G(\mathbf{x}_{Qi}) \, |J_{il}| \quad \text{(26)} \]

\( n_{ct} \) is the number of the curve cells that are used to discretize boundary \( \Gamma_t \), and \( n_{gt} \) is number of Gauss points used in a sub-curve, \( J_{il} \) is the Jacobian matrix for the curve integration of the sub-boundary \( l \) for the Gauss point at \( \mathbf{x}_{Qi} \).

B. Nodal integration

In Gauss quadrature a global background cell structure has to be used, this fact made the method not truly meshless. To avoid the use of background cells Beissel and Belytschko [19] have proposed a nodal integration procedure based on a strain smoothing stabilization to eliminate spatial instability in nodal integration. This technique of integration is based on the substitution of the displacement gradient at a node \( \mathbf{x}_i \) by averaging the displacement gradient over a cell accompanying that node [20]:

\[ \nabla \mathbf{u}^h(\mathbf{x}) = \int_{\Omega_k} \mathbf{u}^h(\mathbf{x}) \, d\Omega \quad \text{(27)} \]

\( \mathbf{W}(\mathbf{x}_k - \mathbf{x}) \) is a smoothing or weight function associated with \( \mathbf{x}_k \), in general we use the following simplest form of the Heaviside-type smoothing function:

\[ \mathbf{W}(\mathbf{x}_k - \mathbf{x}) = \begin{cases} 1/A_k & \mathbf{x} \in \Omega_k \\ 0 & \mathbf{x} \notin \Omega_k \end{cases} \quad \text{(28)} \]

where \( A_k \) is the area of smoothing domain \( \Omega_k \). Equation (27) become:

\[ \nabla \mathbf{u}^h(\mathbf{x}) = \int_{\Gamma_k} \mathbf{u}^h(\mathbf{x}) \, n_k(\mathbf{x}) \, \mathbf{W}(\mathbf{x}_k - \mathbf{x}) \, d\Gamma \]

\[ = 1/A_k \int_{\Gamma_k} \mathbf{u}^h(\mathbf{x}) \, n_k(\mathbf{x}) \, d\Gamma \quad \text{(30)} \]

This nodal integration is based on strain/gradient smoothing technique, this technique is principally used on the smoothed finite element methods (SFEM) [35] [36] [37] [38]. Numbers of SFEM models was developed, because of the types of smoothing domains used [43]. Different smoothing domains created based on cells (cell-based S-FEM (CS-FEM)) [40], nodes (node-based S-FEM (NS-FEM)) [42], edges (edge-based S-FEM (ES-FEM)) [41], and faces (face-based S-FEM (FS-FEM)) [40].

V. ERROR ANALYSIS FOR RADIAL POINT INTERPOLATION METHOD

Two sources of error are noticed in the case of RPIM: the Radial Basis Function interpolation error and the error in calculation of Galerkin weak form. The first error due to the interpolation can be evaluated by the error in fitting different curves and surfaces. The second error is evaluated through the study of convergence rate of RPIM in case of boundary-value problem.

A. RBF interpolation error

In this section, studies on the accuracy of the RPIM shape functions used for curve fitting are conducted. The fitting of functions is based on the nodal function value sets that are generated at regularly as well as at irregularly distributed nodes. The procedure carried out for curve fitting is: first we create a set of field nodes in the domain where the function is to be fitted, then for a given test node \( x \) (usually different from the set of field nodes) where the function is to be fitted, we choose \( n \) nodes in the influence domain of \( x \), now we can construct RPIM shape functions, and finally, using these shape functions we can calculate the function value at \( x \) and compare it with the real value.

The interpolation error of curve fitting at point \( \mathbf{x}_i \) is measured by:

\[ e_i = 1/n \sum_{j=1}^{n} \left| \frac{\tilde{f}_i - f_i}{f_i} \right| \quad \text{(31)} \]
Table I: Radial Basis Functions with global support used in this study.

<table>
<thead>
<tr>
<th>RBF</th>
<th>Equation</th>
</tr>
</thead>
<tbody>
<tr>
<td>MQ</td>
<td>$r_i(d_i) = \left(d_i^2 + (\alpha_c d_c)^2\right)^\eta$</td>
</tr>
<tr>
<td>Exp</td>
<td>$r_i(d_i) = \exp\left[-\alpha_c \left(\frac{d_i}{d_c}\right)^2\right]$</td>
</tr>
<tr>
<td>TPS</td>
<td>$r_i(d_i) = d_i^n$</td>
</tr>
</tbody>
</table>

where $f_I$ is the true function value at point $i$ and $\tilde{f}_I$ is the fitted function value of the function $f$ at point $i$ in the influence domain.

To construct the RPIM shape function we have to compute the inverse of the matrix $G$ (Eq.16), the numerical inversion of this matrix $G$ affects the accuracy of interpolation [23, 24]. Therefore, we add a second indicator to the interpolation error which is the condition number of matrix $G$.

1) Shape parameters analysis: In this section we will see how the shape parameters affect RPIM shape function. Because there is no rule governing the rational choice of the RBF parameters, this theme was and stay a problematic theme when the interpolation by the radial basis functions are used. Several works was made in this axis, we can see how the shape parameters affect RPIM shape function. Through figures 2 to 9, conclusions can be summarized in the following points:

$$r_i(d_i) = \left(1 - \frac{d_i}{\delta_c}\right)^5 \left(8 + 40 \frac{d_i}{\delta_c} + 48 \frac{d_i^2}{\delta_c^2} + 25 \frac{d_i^3}{\delta_c^3} + 5 \frac{d_i^4}{\delta_c^4}\right)$$

\[ (32) \]

The evaluation procedure can be prescribe as follow: for a fixed number of nodes and sampling points, and for a range of RBF shape parameters, we calculate the interpolation error of curve fitting for function defined as:

$$f(x, y) = \sin\left(\sqrt{x^2 + y^2}\right)$$

\[ (33) \]

For this example we choose a large influence domain that include all nodes of the problem, in order to study only the effect of the choice of the FBR parameters, and once good parameters are selected, we can pass to the choice of dimension of influence domain. All sets of nodes patterns (Fig. 1) are employed in order to evaluate the capacity of RPIM to deal with problems where nodes distribution are irregular.

Figure 1: The three sets of nodes patterns.

A total of 100 points are defined as evaluation points. In this study we use three type of RBF with global support: Multiquadric (MQ), Gaussian (EXP), Thin Plate Spline (TPS) (Tab. 1), in addition of an RBF with compact support (Wu-C2) (Eq.32), with the use of polynomial terms (Eq. 1).
1) In order to find the good parameters of the FBR, it would be necessary to find the just balance between the error of interpolation and the condition number of matrix $G$.

2) Singular values found on the two basis MQ and TPS are resulting from the bad conditioning of the matrix $G$ for these values.

3) Both Gaussian (EXP) and Wu-C2 basic functions present no singular behavior for all shape parameters values.

4) We can notice through figures 6 to 9, that the way nodes are distributed has no significant effect on the quality of RBF interpolation.

2) Dimensions of the influence domain: We suggest now to study the effect of the influence domain size (with circular shape) on the accuracy of RBF interpolation. For this, the domain of the problem is discrétised by 529 nodes distributed in an irregular manner. By increasing gradually the size of the influence domain, the number of nodes inside this domain will increase, then we will proceed to the calculation of the interpolation error and the condition number of matrix $G$.

The obtained results are illustrated in figures (10, 11) when we can draw the following conclusions:

1) The condition number of matrix $G$ grows systematically with the increase of the dimensions of the influence domain, thus more the number of nodes inside this domain increases more the matrix $G$ will be badly conditioned.

2) The interpolation error does not improve systematically by increasing the number of nodes inside the influence domain, in our opinion, this is due to two factors, the first one is that more the number of nodes implied in the interpolation is important more the number of computational arithmetic operations is important, this engenders numerical calculation errors, the second is that more the number of nodes is important more the condition number of the matrix $G$ is larger this means that the matrix is ill-conditioned and its inversion will cause bigger error.

3) Larger influence domain means higher number of nodes included in the interpolation, and it implies the increase on the CPU operations (figure [12]), we note that the meshless code is programmed in Matlab and the sim-
ulations are performed on a i5-2.5 GHz computer.

4) From this analysis we recommend that 6 to 25 nodes in the influence domain generate good result and a bigger or smaller number of nodes would lead to larger numerical error.

5) We can mention a technique used in [2] [33] [34], which is based on the concept of the natural neighbor for the construction of the influence domain, the interval recommended by the author for the number of nodes inside (first and second order natural neighbors) Joined what is previously mentioned.

![Figure 10: Effect of influence domain dimensions on Interpolation error with different types of RBF.](image1)

Figure 10: Effect of influence domain dimensions on Interpolation error with different types of RBF.

![Figure 11: Effect of influence domain dimensions on condition number of matrix $G$ by different types of RBF.](image2)

Figure 11: Effect of influence domain dimensions on condition number of matrix $G$ by different types of RBF.

B. Numerical experiments of 2-D solid mechanics problems

1) Error index: The error due to the calculation of Galerkin weak form should be different from that used to evaluate only RBF interpolation over scattered data.

A relative error of displacements is defined as follows : 

\[ e_{dep} = \frac{\| u - \tilde{u} \|_{L^2}}{\| u \|_{L^2}} = \frac{\left( \int_{\Omega} (u - \tilde{u})^T (u - \tilde{u}) \, d\Omega \right)^{1/2}}{\left( \int_{\Omega} u^T u \, d\Omega \right)^{1/2}} \]  

where $\tilde{u}$ and $u$ are displacements computed by the RPIM and the exact analytical solution, respectively.

The error of energy is defined as follows:

\[ e_{Edef} = \frac{\left( \int_{\Omega} \frac{1}{2} (\varepsilon - \tilde{\varepsilon})^T c (\varepsilon - \tilde{\varepsilon}) \, d\Omega \right)^{1/2}}{\left( \int_{\Omega} \frac{1}{2} \varepsilon^T c \varepsilon \, d\Omega \right)^{1/2}} \]  

where $\tilde{\varepsilon}$ and $\varepsilon$ are strain tensors obtained from the RPIM and the exact analytical closed-form solution, respectively.

In order to evaluate the convergence rates of RPIM methods, it is necessary to define a characteristic length $"h"$. For a grid of triangular T3 background cells:

\[ h = \sqrt{\frac{2 A_{\Omega}}{N_c}} \]  

For a grid of quadrilateral Q4 background cells :

\[ h = \sqrt{\frac{A_{\Omega}}{N_c}} \]  

where $A_{\Omega}$ is the area of the problem domain and $N_c$ the number of integration cells.

2) Cantilever beam problem: In this test the accuracy and convergence of combination of shape functions and integration schemes is examined on the problem of a cantilever beam. This problem is widely used to select the optimal parameters of RPIM [29], [30], [31], [32]. Almost all reported conclusions are obtained from this benchmarking numerical experiment.

Consider a cantilever beam shown in Fig[13]. The beam is fixed at the left end and subjected to a parabolic traction force at the free end.

The analytical solution is defined as follows [44]:

\[ u(x, y) = -\frac{P y}{6EI} \left[ (6L - 3x) x + (2 + v) \left( y^2 - \frac{D^2}{4} \right) \right] \]  

\[ v(x, y) = \frac{P}{6EI} \left[ 3vy^2 (L - x) + (4 + 5v) \frac{D^2x}{4} + (3L - x) y^2 \right] \]

(38) (39)
where the moment of inertia \( I \) of the beam is given by 
\[ I = \frac{D^3}{12}. \]

The corresponding stresses are:

\[
\sigma_{xx}(x, y) = -\frac{P(L - x)y}{I} \quad (40)
\]
\[
\sigma_{yy} = 0 \quad (41)
\]
\[
\tau_{xy}(x, y) = \frac{P}{2I} \left[ \frac{D^2}{4} - y^2 \right] \quad (42)
\]

The beam parameters are taken as \( P = -1000N, \ E = 3 \times 10^7 \text{MPa}, \ \nu = 0.3, \ D = 12\text{mm}, \ L = 48\text{mm}. \)

3) **Shape parameters analysis:** For this test, two configurations of nodes will be used: 325 nodes distributed in a regular and irregular way respectively[14]. Only two radial basis functions will be handled: the multiquadric (MQ) and the Gaussienne (EXP). Here circular influence domain circumvents 8 to 16 nodes is used for each Gaussian point. The polynomial term is included for the rbf interpolation. For the gauss integration we use \( 4 \times 4 = 16 \) gauss points for each cell of the set of 288 \((24 \times 12)\) background cells (figure 15).

First, we study the effect of the Multiquadric shape parameters (MQ), results will be illustrated in figures [16] [17].

1) For a uniform distribution of nodes, the optimal error values are registered for \( q \) between 1.5 - 2 and all mentioned \( \alpha_c (\alpha_c = 0.5, 1, 1.5, 2) \), we avoided the value of \( q = 2 \) because it is the particular value which makes the matrix \( G \) singular or strongly ill-conditioned.

2) For an irregular nodes distribution, the optimal value of \( q \) varies according to the value of \( \alpha_c \).

3) The recommended values for the Multiquadric (MQ) are: \( \alpha_c = 0.5 \sim 2 \) and \( q = 0.4 \sim 2.2 \) with the exception of the singular values (integers).

Secondly, we study the effect of the Gaussian shape parameters (EXP), results will be illustrated in figures [18].

1) For a uniform node distribution, the values of the energy error remain relatively invariant for \( \alpha_c > 0.03 \).

2) For an irregular nodes distribution, the behavior of the error according to \( \alpha_c \) is almost the same that for the uniform configuration.

3) The recommended values for Gaussian (EXP) are: \( \alpha_c = 0.2 \sim 1 \).

4) **Dimensions of the influence domain:** In this analysis, we adopt a configuration of 975 \((39x25)\) nodes uniformly distributed, the same configuration is used for quadrilateral background cells (figure [19]). four configurations of shape parameters for the FBR will be used: RBF-MQ with: \( q = 1.3 \)
and $\alpha_c = 0.5$ and $q = 1.9$ and $\alpha_c = 1.5$, RBF-EXP with: $\alpha_c = 0.2$ and $\alpha_c = 0.6$. The number of Gauss points is maintained to 16 points by cell.

1) Large size of influence domain does not imply systematically a good precision, quite the opposite, number of arithmetic operations required for the calculation increase, what would have consequence on increasing the CPU time (figure 21).

2) The number of nodes recommended to reach a good quality is of $9 \sim 30$ nodes inside the influence domain.

5) **Number of Gauss points:** To analyze how many Gauss points by cell used on the Gauss integration technique affect the accuracy of RPIM, we have to fix these parameters: for nodal discretization and background cells we use the previous uniform configurations Fig.14 and 15, two radial basis functions are considered: the RBF-MQ with $q = 1.9$ and $\alpha_c = 1.5$, the RBF-EXP with $\alpha_c = 0.2$, the influence domain being determined by 6-12 nodes inside.

6) **Effect of the integration technique:** The objective of this section is the study of two numerical integration techniques: the classical Gauss integration and the Stabilized conforming nodal integration (SCNI).

In all what follows, the Radial basis function chosen is the Gaussian (EXP) with $\alpha_c = 0.2$, with addition of 1$^{st}$ order polynomial terms, influence domain being determined with 8-16 nodes inside. A total of $6 \times 6$ Gauss points are used for the classic Gauss quadrature.

five set of regularly distributed nodes pattern and in the same time integration cells are used for this exemple: 1150, 798, 520, 360, 284.
We notice, first, that the convergence rates of meshfree methods (RPIM, SRPIM) are better than those obtained by the FEM, this is due to the higher order of RBF interpolation comparing with the linear interpolation used in FEM triangular element, secondly, among RPIM meshfree methods the one who presents a better rate of convergence is SRPIM, who possesses the advantage of using a nodal stabilized integration with regard to the classic RPIM which use Gauss integration.

Figure 23: Relative error in displacement calculated for FEM, RPIM, SRPIM.

Figure 24: Relative energy error calculated for FEM, RPIM and SRPIM.

C. Infinite plate with a circular hole

We consider here a plate with a central circular hole with radius \( a = 1 \text{m} \) subjected to a unidirectional tensile load of \( F = 1 \text{N/m} \) in the x-direction, due to symmetry, only the upper right quadrant of the plate is modelled as shown in Figure 25. Symmetry conditions are imposed on the left and bottom edges. The inner boundary is traction free. Plane strain conditions are assumed, and the problem constants are: \( L = 5 \text{m}, a = 1 \text{m}, E = 10^3 \text{N/m}^2, \nu = 0.3, P = 1 \text{N/m}. \) analytical solution of these problem [44]:

\[
\begin{align*}
    u_1 &= \frac{a}{4\mu} \left[ \frac{r}{a} \left( \kappa + 1 \right) \cos \theta \right. \\
    &\quad + 2 \frac{a}{r} \left( (1 + \kappa) \cos \theta + \cos 3\theta \right) - 2 \frac{a^3}{r^3} \cos 3\theta \left. \right] \\
    u_2 &= \frac{a}{8\mu} \left[ \frac{r}{a} \left( \kappa - 3 \right) \sin \theta \right. \\
    &\quad + 2 \frac{a}{r} \left( (1 - \kappa) \sin \theta + \sin 3\theta \right) - 2 \frac{a^3}{r^3} \sin 3\theta \left. \right] \\
\end{align*}
\]  

where \((r, \theta)\) are the polar coordinates with \(\theta\) measured counterclockwise from the positive x axis, \(\mu = E/(2(1+\nu))\) et \(\kappa = 3 - 4\nu\).

The analytical solution for the stresses of an infinite plate is:

\[
\begin{align*}
    \sigma_x(x, y) &= 1 - \frac{a^2}{r^2} \left\{ \frac{3}{2} \cos 2\theta + \cos 4\theta \right\} + \frac{3a^4}{2r^4} \cos 4\theta \\
    \sigma_y(x, y) &= -\frac{a^2}{r^2} \left\{ \frac{1}{2} \cos 2\theta - \cos 4\theta \right\} - \frac{3a^4}{2r^4} \cos 4\theta \\
    \sigma_{xy}(x, y) &= -\frac{a^2}{r^2} \left\{ \frac{1}{2} \sin 2\theta - \sin 4\theta \right\} + \frac{3a^4}{2r^4} \sin 4\theta \\
\end{align*}
\]

The displacement boundary conditions are given by:

on the edge of \(x = 0\) : \(u_x = 0\), on the edge of \(y = 0\) : \(u_y = 0\).

As in the previous example, five nodes configurations will be used for this problem also, the total number of nodes used is: 169, 289, 625, on 1089 and 1681. Figures 26, 27, illustrate convergence rates of results obtained from RPIM, SRPIM and FEM. We notice as well in this problem that the convergence rates obtained from meshless methods, are better than those obtained by the FEM, and among the RPIM methods, SRPIM presents a better precision in term of displacement error and in energy error, what confirms the quality of the SCNI integration.

Figure 25: Geometry of the infinite plate with a hole problem.
Figure 26: Relative error in displacement calculated for FEM, RPIM, SRPIM.

Figure 27: Relative energy error calculated for FEM, RPIM and SRPIM.

D. Distortion analysis

The example treated in this test is similar to the precedent test of cantilever beam (Figure 13), but with a different geometry and load (Figure 28), this geometry will be discretised by various nodes configurations (Figure 29).

The Radial basic function chosen is the Gaussian (EXP) with \( \alpha_c = 0.2 \), influence domain being determined with 8-16 nodes inside. A total of \( 6 \times 6 \) Gauss points are used for the classic Gauss quadrature.

The exact solution calculated on the field of stress:

\[
\sigma_{xx}(x) = \frac{3}{2} y, \quad \sigma_{yy}(x) = 0, \quad \sigma_{xy}(x) = 0 \quad (48)
\]

The exact solution calculated on the field of strain:

\[
\epsilon_{xx}(x) = \frac{3 E (1 - \nu)}{2 (1 - \nu)(1 - 2\nu)} y, \quad \epsilon_{yy}(x) = \frac{3 E\nu}{2 (1 - \nu)(1 - 2\nu)} y, \quad \epsilon_{xy}(x) = 0 \quad (49)
\]

We notice in Figures 30 and 31 that for the first nodes configuration distributed in a regular way (configuration (1)), all RPIM methods gives satisfactory results and better than those obtained by finite element method (with linear triangular element mesh).

For the last nodes configuration, the most severe twisted configuration, and in spite of the fact that we use an influence domain guaranteeing a minimum number of nodes inside the latter, we notice that all RPIM methods diverges but by different degrees, the method which seems the least affected by this distortion is the S-RPIM (with SCNI technique). In summary, the more the distortion of the meshing of nodes is big, the more the error in displacement and in energy increases, however the methods using the SCNI integration technique are less affected by this degradation than those using the classical Gauss integration.

VI. Conclusion

In this paper a numerical analysis was performed on the RPIM meshless method with two integration schemes. The performance in the RBF shape function and in linear elasticity. It was shown that the Radial basis interpolation was important to the global accuracy of the RPIM. It was found that the condition number of the matrix \( \mathbf{G} \) heavily affects the accuracy of interpolation. Shape parameters also had important effects on the condition number. A range of good shape parameters should balance the accuracy and the condition number. The following notes can be retained: Error was large when shape parameters takes singular values \( (q = \text{integer values for RBF-MQ}) \). Through numerical experiments, a range of shape parameters was found. Parameters from this range can give
RELATIONS

REFERENCES


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